# **Waste Tire and** Shingle Scrap/Bituminous **Paving Test Sections**



Willard Munger Recreational Trail **Gateway Segment** 

**Interim Report** 





A Cooperative Study Performed by Minnesota Pollution Control Agency Minnesota Department Of Natural Resources Minnesota Department of Transportation



REPORT DOCUMENTATION PAGE	1. Report No. MN/RD - 91/06	2.	3. Recipient's Accession No.
4. Title and Subtitle Waste Tire & Shingle	Scrap Bituminous Paving Te	st Sections On The V	5. Report Date February, 1991 6.
	rail Gateway Segment		6.
7. Author(s) Curtis M. Turgeon, Re	search Project Engineer		8. Performing Organization Rept. No. 9PR6002
9. Performing Organization Nar Physical Research Sec	ction		10. Project/Task/Work Unit No.
Office of Materials and Minnesota Department			11. Contract(C) or Grant(G) No. (C)
1400 Gervais Avenue	•		(G)
Maplewood, MN 5510  12. Sponsoring Organization Na Minnesota Department Minnesota Polution Co	me and Address t of Transportation, and;		13. Type of Report & Period Covered Interim Report 1990-1991
520 Lafayette Road St. Paul, Minnesota 5	5109		14.
15. Supplementary Notes Minnesota Departmen	t of Natural Resources		

#### 16. Abstract (Limit: 200 words)

500 Lafayette Road

The need to reduce our states dependence on land fills resulted in a unique cooperative venture by three state agencies. A partnership was forged between the Minnesota Pollution Control Agency (MPCA), the Minnesota Department of Natural Resources (DNR) and the Minnesota Department of Transportation (Mn/DOT) to investigate the use of recycled tire rubber and processed asphalt shingle scrap. The result is a two mile section of the Willard Munger Recreational Trail in St. Paul, constructed with asphalt paving mixtures which contain varying percentages of recycled tire rubber and shingle scrap.

Special bituminous mix designs were formulated using 3% rubber, 6% rubber, 3% rubber with 6% shingles and 9% shingles. The mixtures containing rubber did not exhibit acceptable mix characteristic values under present Mn/DOT bituminous specifications. The shingle-only mix met specifications and yielded an economic advantage of decreasing the asphalt cement demand of the mix.

Conventional mixing and paving equipment was utilized for construction. This application appears to be a viable alternative containing rubber increased from 35% to 50% over the cost of the conventional mixture. Since the use of shingle scrap was negotiated by the private companies involved, no comparable cost data is available.

17. Document Analysis a.Descriptors Asphalt Concrete Pavement Waste Tires Shingle Scrap Bicycle Trail

b.Identifiers/Open-Ended Terms

c.COSATi Field/Group

18. Availability Statement No restrictions. This document is available through the National Technical Information Services,	19. Security Class (This Report) Unclassified	21. No. of Pages 30
Springfield, VA 22161	20. Security Class (This Page) Unclassified	22. Price

# Waste Tire and Shingle Scrap/Bituminous Paving Test Sections on the Willard Munger Recreational Trail Gateway Segment

# **Interim Report**

Construction Report by
Curtis M. Turgeon
Research Project Engineer
Minnesota Department of Transportation

A Cooperative Study Performed by
Minnesota Pollution Control Agency
Minnesota Department Of Natural Resources
Minnesota Department of Transportation

#### ABSTRACT

The need to reduce our states dependence on land fills resulted in a unique cooperative venture by three state agencies. A partnership was forged between the Minnesota Pollution Control Agency (MPCA), the Minnesota Department of Natural Resources (DNR) and the Minnesota Department of Transportation (Mn/DOT) to investigate the use of recycled tire rubber and processed asphalt shingle scrap. The result is a two mile section of the Willard Munger Recreational Trail in St. Paul constructed with asphalt paving mixtures which contain varying percentages of recycled tire rubber and shingle scrap.

Special bituminous mix designs were formulated using 3% rubber, 6% rubber, 3% rubber with 6% shingles and 9% shingles. The mixtures containing rubber did not exhibit acceptable mix characteristic values under present Mn/DOT bituminous specifications. The shingle-only mix met specifications and yielded an economic advantage of decreasing the asphalt cement demand of the mix.

Conventional mixing and paving equipment was utilized for construction. This application appears to be a viable alternative to landfilling these materials. However, costs for the mixtures containing rubber increased from 35% to 50% over the cost of the conventional mixture. Since the use of shingle scrap was negotiated by the private companies involved, no comparable cost data is available.

#### ACKNOWLEDGEMENTS

The author would like to acknowledge the contributions of the following individuals and their organizations. An immense amount of cooperation and coordination was required to implement this concept.

Tom Ask, Bituminous Roadways Inc.
Robert Englestad and Jerry Buehler, Trash Depot Inc.
Roland Doerr, Certainteed Shingles Inc.
Mike McCormick, J.L. Shiely Inc.
David Omann and Jim Omann, Omann Brothers Inc.
Frank Wallner, MPCA
Tim Peterson, MDNR
Greg Murray MDNR
Tom Danger MDNR
Rick Walkosz, Mn/DOT
Roger Olson Mn/DOT
Doug Schwartz, Mn/DOT

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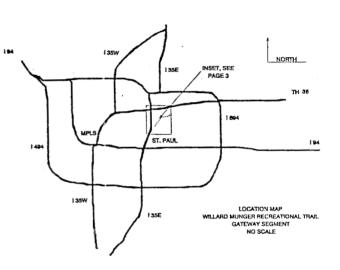
#### Background.

The need to reduce our states dependence on land fills resulted in a unique cooperative venture by three state agencies. A partnership was forged between the Minnesota Pollution Control Agency (MPCA), the Minnesota Department of Natural Resources (DNR) and the Minnesota Department of Transportation (Mn/DOT) to investigate the use of recycled tire rubber and processed asphalt shingle scrap. The result is a two mile section of the Willard Munger Recreational Trail in St. Paul constructed with asphalt paving mixtures which contain varying percentages of recycled tire rubber and shingle scrap.

The location of the Gateway Segment, which contains the test sections, is shown in Figure 1. The test sections contain the following proportions of waste materials:

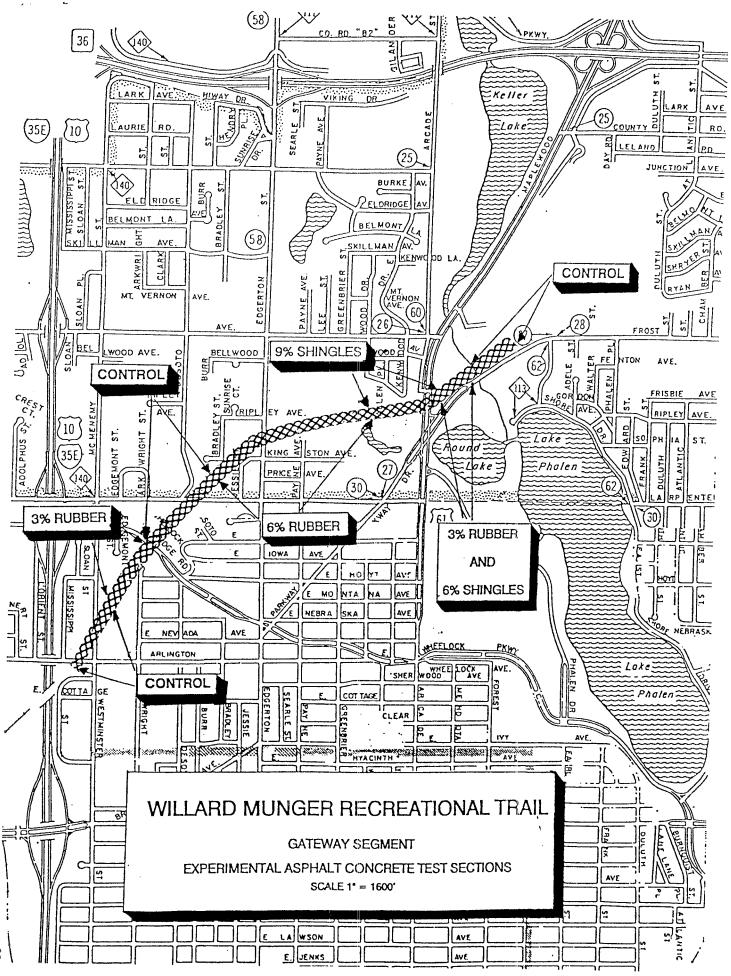
- 3% Rubber
- 6% Rubber
- 3% Rubber and 6% Shingle Scrap
- 9% Shingle Scrap

( Percent by weight of the mineral aggregates.) Asphalt mixture utilizing only standard aggregates was also used as a control mix for comparison purposes. Locations of the test and control sections are shown on the following page.



Since 1985, when the landfilling of waste tires became illegal, there have been ongoing efforts to find viable markets for waste tire products. Presently, most of Minnesota's waste tires are shredded and consumed as industrial boiler fuel. The use of ground tire rubber in asphalt paving mixes is not new. Past experiences by Mn/DOT and other agency's had shown them to be constructable. However, highway experiences demonstrated less room for error when placing rubber mixtures and, when errors occurred, catastrophic failures ensued. Rubber mixtures also require a higher percentage of asphalt cement. Since asphalt cement is by far the most expensive ingredient in an asphalt paving mixture, higher asphalt demand equates to a significant rise in construction cost.

The concern over potential failures led to the concept of testing the mixtures on recreational trails instead of on highway construction. Using the trail allowed a more bold approach in formulating the mixture designs since reconstruction of failed test sections would not pose the traffic control problems and other costs associated with highway testing. However, the trail pavement is subjected to exactly the same mixing, paving and, most importantly, environmental factors common to all asphalt paving.



The use of shingle scrap was identified as a means to reduce the mixtures asphalt demand and make the use of rubber more economical. Organic and fiberglass shingles, the two types presently produced in the United States, contain approximately 30% and 19% asphalt cement, respectively. Ends of runs, samples, off color shingles and tabs create approximately 5% waste during the manufacturing process. This waste is presently landfilled. It is important to note that the shingle scrap was obtained directly from a shingle manufacturing plant. It did not include waste removed during reroof construction. While utilization of reroof waste has merit, problems with uniformity, removal of nails and material separation preclude its use at this time.

#### Laboratory Bituminous Mix Design.

Previous studies and background testing had shown that mixes containing ground rubber display lower Marshall stability (a standard measure of resistance to deformation or strength) and higher air void content than conventional mixtures. Upon awarding the contract to the low bidder, Bituminous Roadways, samples of standard aggregates were obtained from their stockpiles. It is essential that the actual aggregates to be used on any bituminous project are used in the trial mix testing.

The DNR specifications called for the use of standard Mn/DOT mix type 2341. Trail mix lab work performed in the Mn/DOT Central Lab yielded the following mix designs.

% Agg	% Rubber	% Shingles	% Asphalt	Stab.	% Air Voids
100	0	0	5.0	1560	4.2
97	3	0	6.5	192	5.7
93	6	0	7.7	50	9.1
91	3	6	5.9	408	5.3
91	0	9	3.0	2464	3.3

<sup>%</sup> Aggregate + %Rubber + % Shingles = 100% weight of dry mineral materials.

Some stability and air void data was interpolated from actual test results. Quantities shown are for comparison purposes only.

Mix design sheets are in Appendix A of this report.

<sup>%</sup> Asphalt = % by weight of total mixture (asphalt cement
and mineral materials)

The Mn/DOT bituminous specification type 2341 for 1990 required a minimum Marshall stability of 1000 lbs. and a maximum of 3000 lbs with a targeted air void content of 4.0.

All mixes contained the same combination of natural aggregates. The control mix met type 2341 gradation requirements but was formulated to be on the coarser side of the tolerances. Experience with other aggregates had shown that when rubber is added, relatively coarser aggregates yield higher stabilities. It is theorized that larger stones maintain better aggregate interlock as the smaller less dense rubber particles fill the gaps. Even with this slight beneficial influence, rubber-only mixtures exhibited extremely low stability values.

The stability values for the rubber and shingle mix were somewhat better, but still failed to meet specification. A notable decrease in asphalt demand, 0.6 percent, was also demonstrated. The shingle-only mix met specification and demonstrated a significantly higher Marshall stability than the control mixture. The angular granules and relatively hard asphalt cement contributed by the shingle scrap are potential sources of this increase in stability. The decrease of 2.0 percent in asphalt demand displayed has important potential economic benefits.

#### Pre-Construction.

A great deal of planning was required to bring together the information and materials to make this project work. The DNR prepared the plans and specifications, with assistance from Mn/DOT. The MPCA contracted with the Trash Depot Inc of Moorhead, Minnesota to produce and deliver the ground rubber. The use of shingle scrap was coordinated by the J.L. Shiely Company. This including locating a shingle scrap source; Certainteed Inc of Shakopee, Mn; selecting a processor/grinder, the Omann Brothers of St. Michael, Mn; and working with the paving contractor, Bituminous Roadways. Performing the mix design testing was provided by Mn/DOT.

#### Construction.

This section of trail was placed on abandoned Soo Line Rail Road right of way. The inplace track bed was reshaped as needed and a 4" thick crushed concrete base was placed and compacted.

A batch-type plant was use to prepare the mixture. All waste materials were introduce through the plant's recycled asphalt pavement inlet. No recycled asphalt pavement was utilized on this project. Higher mixing temperatures (30-40 degrees F higher than the normal 290 F) and slightly longer mixing times were utilized to foster better rubber/aggregate coating. Since only one inlet was available for waste product introduction, premixing of the rubber and shingles for the rubber-shingle mix was required. This was accomplished with a front-end-loader and truck scale. The

contractors versatility and previous experience with rubber athletic tracks led to relatively smooth production with few unexpected plant problems.

The weather during construction was hot and humid with clear skies and highs in the 90's F. The 12 foot wide, 2.5 inch thick mat was place in one paver pass. A steel wheeled roller provided compaction with a second smaller steel wheeled roller creating the finished surface. The use of pneumatic tired roller was no recommended due to potential rubber pickup problems. A few 2-3 inch diameter clumps of shredded shingles appeared during paving. While the clumps posed only a minor problem, a process to break up or remove them should be adopted.



#### Performance.

The surface texture immediately after construction was somewhat open and porous. Much of this was due to the coarse natural aggregate gradation used in hopes of gaining stability. It appears that what little was gained in stability was sacrificed in terms of surface texture.

Shortly after construction some loss of rubber particles from the surface occurred. As yet, this phenomenon has been minimal and has not significantly effected the surface texture.

While the shingle-only mixture also exhibited a relatively open surface texture due to the coarse natural aggregates, it is performing satisfactorily to date.

#### Laboratory Analysis.

Core sample were taken from each test section. The following parameters were evaluated in the Mn/DOT lab.

Mix Type	Density (Bulk) lbs/cu.ft	Split Tensile Strength psi. avg. / range	Inplace Air Voids (Rice)	%AC	AC PEN
Control	141.7	70 / 64-76	9.0	5.3	52
3% Rubber	128.8	42 / 28-50	12.6	6.3	76
6% Rubber	122.7	30 / 29-31	13.0	7.8	111
3% Rubber 6% Shingles	129.6	40 / 34-48	12.6	7.3	55
9% Shingles	130.5	37 / 31-48	16.1	5.4	34

%AC = Percent of extracted asphalt cement by weight mix. AC PEN = Penetration of recovered asphalt at 77 F.

(Penetration is a relative measure of the stiffness of the asphalt cement. The term "recovered" refers to the process of washing the asphalt from the aggregate with a solvent. The asphalt and solvent are then separated and the asphalt is tested.)

One should exercise discretion when comparing the above data since the tests are formulated for standard asphalt-aggregate mixtures.

The control mix exhibited relatively standard results. The tensile strength is somewhat low but this may be due to the coarseness of the aggregates. The grade of asphalt cement used was 120-150, which means its penetration prior to mixing fell between 120 and 150. The heat applied during mixing causes the asphalt to stiffen, hence the recovered (after mixing and placement) penetration of 52.

The rubber-only mixtures have in general lower densities, lower tensile strengths, higher air voids and have asphalt contents close to the prescribed contents in the mix design formulation. The penetration values seem to indicate some resistance to the normal asphalt hardening due to heating and mixing. The rubber may be soaking up and "hiding" the asphalt from the heat. However the rubber may be reacting with the solvent in the extraction/recovery process and tainting the results. Further studying is needed to clarify this point and what effect it has on mix properties.

The rubber-shingle mix yielded results similar to the rubber-only mixes with the notable exception of the recovered penetration. The asphalt cement contributed by the shingles is relatively hard. (low penetration) Therefore the combination of the standard asphalt and

shingle asphalt creates a harder binder.

The effect of the stiff shingle asphalt is more apparent in the shingle-only mix. The penetration of the shingle-only mix is lower than the control mix. Stiffer binders can be more susceptible to low temperature cracking. Cracking of the shingle-only mix should be monitored closely. Inplace air voids are higher than expected as well. The stiffness of the asphalt cement may also inhibit compaction.

#### Cost.

Precise costs are difficult to establish due to the wide variety of organizations contributing to the project. The MPCA contract, which included purchase and delivery of the ground rubber, yielded the following totals:

Tons of rubber delivered: 38.0
Price per ton: \$ 125.00
Total Contract: \$4,750.00

The unit prices in the DNR contract for mixing and placing the control and rubber mixes were as follows:

Control mix \$ 3.60 /sq yd 3% Rubber \$ 4.40 /sq yd 6% Rubber \$ 4.50 /sq yd

When the rubber purchase and delivery is factored into the mixing and placing costs the total costs become:

Control mix \$ 3.60 /sq yd 35 Rubber \$ 4.85 /sq yd 6% Rubber \$ 5.41 /sq yd

One can see that the total cost for the rubber mixtures is 35% to 50% higher than the control mix.

The concept of using shingle scrap developed after the above contracts were awarded. All cost for transport, processing, mixing and placing were negotiated between the private companies involved. All materials and processing were provided to the State at no cost. It is possible that the savings from the decrease in asphalt demand would offset any handling or processing costs. Depending upon the price of asphalt cement, shingle scrap use may actually decrease the bituminous mixes total cost.

#### Conclusions.

- 1. This project has shown bituminous trail construction with two waste products, ground tire rubber and shredded shingle waste, to be a viable alternative to landfill disposal.
- 2. Laboratory characteristics of bituminous mixtures containing

ground tire rubber did not favorably compare to the control mix or to the Mn/DOT specifications. Rubber mixtures exhibited high air void contents, low Marshall stabilities and high asphalt cement demand.

- 3. Improved Marshall stability can be achieved by using a coarser natural aggregate gradation. Unfortunately this causes the pavement surface to be open or porous in appearance.
- 4. Ground shingle scrap effectively reduced asphalt demand and increased Marshall stability.
- 5. Analysis of core test section samples removed after construction displayed low density, low tensile strength and high air voids when compared to the control mix. Mixes containing rubber had higher recovered asphalt penetration and mixes containing
  - shingles had lower recovered asphalt penetrations when compared to the control mix.
- 6. The total cost for using the 3% and 6% rubber-only mixes was 35% and 50% higher, respectively, than the control mix. No expense for the use of shingles was born by the State, therefore these costs are not reported.

#### Recommendations.

- 1. The test sections should continue to be monitored for surface abrasion, cracking and general performance.
- 2. The natural aggregate selection and mix design process for rubber bituminous mixes for trails should focus equally on standard laboratory data (Marshall stability, air void content, asphalt demand) and potential surface texture/porosity.
- 3. A cost comparison/analysis should be undertaken to determine if this is a cost effective/competitive means of waste tire disposal.
- 4. Further testing on the use of shingle scrap in bituminous mixes is warranted.

# APPENDIX A BITUMINOUS TRIAL MIX RESULTS EXTRACTION AND GRADATION RESULTS



Date: 8/23/90 Phone: 612-779-5614 FAX: 612-779-5580

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Date: <u>8/23/90</u>
Phone: 612-779-5614
FAX: 612-779-5580

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Cont. Imm,		P	162.1	and the second section of			~.	Р	185.4			
<u>Sa</u> Imm		Q	11668					G (	11526		<b></b>	
Sa Volume	<u>M - Q</u>	R	849.3					R	862.3	<del> </del>	<b></b>	_
Max.Sp.G.	M/R	S	2.374			23	74	S	2.337			233
Rice Voids		Т				8.	2	T				5.7
TILE ADIO?	Abersions	1		2.580	5	Abi	25.		2.57	3	<del></del>	Abras
Pan,Agg. Dry	Torget	d		AC ratio	b/c		223	P				210.6
Pun Dry	(set 6)	е		Agg ratio	100-b/n			7				
Agg Dry	Dey Actual	f			P + q			r				1062
Pail AC Dry	55D	g		TM Sp.G.	100/r	<u> </u>		S				1074.
Fail AC Imm	Imm	<u>h</u>		<b>NE MIX</b>	$(E/s) \times 100$		i	<u> </u>			<del> </del>	511.5
Pail, AC Volume	ORigh	<u>  [</u> -	<u> </u>	Vol,AC	Exp	-		u			<del> </del>	1140.
Pail, AC, AggDry	Final	<del>  <u>i</u> </del>		V.M.A.	u + y	-		<b>y</b> .		<del></del>	<del> </del>	893. 247. (
RijACAgy Imm		k		V.t. Agg.	(4, )×100	<b>}</b>		₩			<del> </del>	
PhilACAGOVCI.		ın	+	B     Voids	100-1			У				21.4
Agy. Volume	70 £055	r.		V0103	<b>†</b>		7,	ali	''			: 8/23
Ags. Sp. G.	/m	1.	1	i	Signed	120	1. h.	all	town		date	: 0/24



Date: 8/23/90

Phone: 612-779-5614 FAX: 612-779-5580

To: _			, En	gineer,				
The r	nix design for Spec is project as follows:	2.346	-Special, M	ixture Type _	41.	5	is hereby appro	ved
S	The second control of	1 1	-		јО [В		ليت	LA
	<del>-</del> -	er:						
	The mix design for Spec. 2.340 - 5 pc id., Mixture Type		120					
	,	110	100   e	3/4"				4
Base								-1
V.	(Card 2)	21		1/2"				4
	The mix design for Spec. 2.740 - Special, Mixture Type		4					
	21	27		ш, —		7	<del></del>	ł
For	S.P.   9							
Leveling		السويبيوسا		<b> </b>	1		<del> </del>	4
	(Card 4)	27					<del>                                     </del>	ł
	41/   W  E  210	10			1			
,, caring	21		7.7	% A. C.	Į	7. 4	8.0	]
For	(Card 5)					MIII.	Max.	
Shoulders			Use 120 / /	50 penetra	tion gra	de asphalt.		
			indica	tes a Marshall	Density	y of	?7 • O PCF	at
		per side.	Course	CMataria)		,	D 4 DD 4	
		O 11						7
	8 %	Solberg -	Nininger	(3/4 Kock)	<u>)                                    </u>			_
				(3/4" //invs	رر	i		-
					<u> </u>			_
	<u> </u>	leash d	epot (	Kubber)	<del></del>	i ·	1397	-
		<i>-</i> ,		ma / /				ا
				,		C. Jungeon/K	2. Wallosz.	
/	1/1x will be Tende	e until	cooled	below 140	20 F.			
			**************************************					
Approve	d by;		Dist		) (2)			
Assistant	Bituminous Engineer							

nDOT TP 24303 A (1-4	18)		Sheet	of	TI	M. No.			<del></del>
					14	VI. 1104	0-9	0192	
			Specific	otiqu No	<u>.</u>	415	Previou	us Mix No.	
	008-0		AB No.			Source		t	ortion
Location Ramsey		1.No.	90392		lberg	<b>?</b>		- 8	
Additive	يح		393		t Kd	ngs		58	
AC Source Ashland AC Grade 120/150	<u> </u>		394	34	ely	Rubber	. e como merco.	28	
AC Sp.G. c 1.017	Í		397	(	<b>331</b>	Nuove	A SE LANGUE AND SEMESTICS AND SECURITY OF	6	
	l loil i	MI T	· ·	,			<b>.</b>	· · · · · · · · · · · · · · · · · · ·	
Tests	LBI.	Mr Ts	Extract		irad		Other		
	MIX	147.6	date	: 8/12		MIX	3 8425	date	
	wt. AC		# Blows	. 50		WI AC D		# 0.	50
Marshall LD	ZOAC	7	7	$\frac{3\nu}{2}$	-	MACIO	2	# Blows	30
Height	0 23%		22932		a	2 18/32	217/32		;
Dry Wght.	A 1090.	5 1094.0	1090.9		Α	1074.5	1070-1	1075.4	
SSD Want.	B 1098.		1100.0		B	1081.2			
	C 564				ĬČ.	553.5	550.2	555.9	
	D 533	1	5429	700	D_	527.7	525.1	526.6	
Bulk Sp.G. AD	E 2043	2648	2009	2046	E	2036	2038	2042	2039
Density Ex 62.3	F		na 1-00.	127-5	F				127.0
Flow	15	10	10		1	//	//	11	
Stability Dial	100	50	50	_,		50	50	50	
Stability Dial chart x ht. Stability				67			_	İ	50
		· · · · · · · · · · · · · · · · · · ·	1		]	<u> </u>			
Container ID	F			h <del></del>		1-			-
Cont. + Sa Dry	K				K				!
Cont. Dry	L 780. M 2020.	7		-	M	776.5			
·	M 2020 . N 1330.4				N	1314.0		· · · · · · · · · · · · · · · · · · ·	
Cont. Imm,	P 196.4		<b>1</b>		Р	193.9			
So Imm N-P	Q 1134.0				Q	1120.1			
So Volume M-Q	R 886.9	3			IR.	900.5			
Max. Sp. G. MR	S 2.279	)	A SECURITION OF VALUE OF A SECURITION OF A SEC	2279	S	2.244			2.244
Rice Voids 100x(S-E)	Ti			10.2	T				9.1
		2.494		Abeas	]	2.498	3	· · · · · · · · · · · · · · · · · · ·	Rhoas
Pon, Ago Dry target	d	AC ratio	b/c	195.6				1948	1915
Pun Dry set 6	e	Agg ratio	(100-b)/n	1173.6	1				1168.8
Agg Dry Day-Actual		TAIS	P + q	1203.0	s r				1205.9
Poil AC Dry 55D	_9 ( h	TMSpG VEMIX	(E/s)× 100			<b>†</b> ———		<del> </del>	531.4
Pail AC Volume Origh	ſ	Vol. AC	Exp	1336.3	u′				1293.0
Bil, AC, Agy Dry Final	j	V.M.A.	u + y	950.0					10900
PailACAgilmm 4055	k	V.F. Agg.	(4, )×100	3863	W.,			ļ	2030
Hail AC Ag; Vol. 1-k	in	Voids	100 - t	28.9	у				157
Agy Volume % Loss	r.	Aoiaz	+	1.7	<del>•</del> //	1 / Dh			5/22
Agj, Sp. G. /m	· · L		signed	4	(L/a	lkos	s on the design of the state of	yate:	0/23





Date: <u>8/23/90</u>
Phone: 612-779-5614
FAX: 612-779-5580

To: _		, Engin	neer,			
The n	nix design for Spec. 2	346 Special, Mixto	ште Туре	415	is hereby a	pprove
S	S.P. 9 A.P. 1 2	11	12	O  B   -  M	I I X FOR	MUL
	Begin with test number:	Formula	Sieve Size		Working Range	
For Base For Binder For Leveling	(Card 1)  4	100 Pe 100 r r c e 96 n t 53 p a s	1 1/2" 3/4" 5/8" 1/2" 3/8" #4 #10		0 0 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
For Wearing For	21 (Card 4) 27 4   /   W   E   3   0   0 21 (Card 5) 27	2Z s 3,9 5.9 Target (New)	#40 #200 % A. C.	81 2	6 6.2	
Shoulders	21 27 Trial Mix No. 0- 90/93			ion grade asph	<b></b>	PCF at
	Proportions	Source of N	Material		BA or BR	#
	57 % Bitus	erg-Ninininger (= minous Radways ( Jy-Larson (+	3/4" Rock ) (3/4" Minus 12" Rock )	<u>)</u>	0- 90392 0- 90393 0- 90394	
	~ 0.112	7	ubber)		0-90397	
	6 % Om:		(ingles)		0-90415	
Rema	urks: Chtain 60# Sa	mole for Maplewo	, ,	6 C. Turque	/R.Waltesz	

DOT TP 24303A(1-	<b>88</b> )		٠	Sheet _	of _	_	TI				
							141	M. No.	0-7	0193	
				Specific	alian	No.		415	Previo	us Mix No	•
Project No. 998 100	8			AB No.				Source		i	portion
Location Kensey		TH'I	Vo.	90392		<u>So/b</u>		)		7	
Additive	,	<u> </u>		393		RIT	Ķď	wes		57	
AC Source Ashland AC Grade 120/15		Application and Property age and application of the con-		394				Larson		27	200 C 200 M 2
AC Grade 120/15 AC Sp.G. C 1.617	۲			397 415	_ I			Rubber Shingles	a or charged as some an .	3	
	<u></u>										
Tests.		B.I. M	r Ts	Extract	ion	G	adı	ation C	Other	<u> </u>	-
		MIXA		date	: 8/	3		MIX	3	date	:8/21
<b>i</b>	522.9	WI. AC		۱ 44 ــ			45.2	WI AC	بدرين المستجهدة المستجهد	•	
MID	5.5	% AC b	1	# Blows	50	2	6.3	% AC b		# Blows	50
Marshall ID	0	219/32	218/32	2/8/32	<del></del>		a	2/32	2/32	2 /32	
Height Dry Wght.	A	1108.9	1109.8	1114.7			A	1031.6	1031-6	1027.2	
SSD Waht.	B	1110.1	1110.8	1117.4			B	1032,0	1032.6		
lmm. Wght.	C	583.0	594.2	597.7			C	568.8	566.3		
Volume B-C	D	515.1	5166	5197			D	463.2	466.3	463.2	
Bulk Sp.G. AD	E	2153	2148	2145	214	9	Ε	2.227	2.212	2.217	2.7.19
Density Ex 62.3	F				133	.9	F	to the total section of the section	<u> </u>	2.2.	138.2
			12	11		$\stackrel{\checkmark}{\dashv}$			17	   17	
Stablis Dia	<del>                                     </del>	12 320	320	380				·20_	430	430	<u> </u>
Stability Dial chart ht. Stability					325		;				633
Stability \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	!	304	307.2	365	545			615	490	1490	<i>53</i> Z
Container ID		A					!	A			!
Cont. Sa Dry	K		•				K				
ont Dry	L	943.1					L	943.1			
ja. Dry K-L	M	2028.0					M	2023.9			
Cont+Solmm		1_08.3					N	1485.0			
Cont. Imm,	1 E	353.2					Ď	353.2		<u> </u>	<u> </u>
Sa Imm N-P Sa Volume M-Q	Q R	1155.1	Chiango				G R	1131.8	<sub>फरा</sub> तरहे		
M	1	7	2311		22	22			t .		
Max. Sp. G. M/R	S	2.323	<del></del>		2.3	7	<u>S</u> _	2.269	1259	<del> </del>	2.2.69
Rice Voids 100x(S-E)	<u> </u>		7.0		7.5	_	T	161	2-7	ļ	2.2
	<del>-</del>	EFF Sp		/	Abr	25		2481	Arg = 2.61	Ý	Abars 212 0
Pan, Aug. Dry Target	d		AC ratio	b/c (100-b)∕n	20.	, ,	P				12720
Run Dry (set 6) Aga Dry Dry Act.	e f		Agg ratio	p + q	1/2/2	(C)	ų r			<del>                                     </del>	16-16-0
	g		TM Sp.G.			$\neg$	S			<u> </u>	
Pail, AC Dry SSD	h		VE MIX	(E/s)× 100	Ī .		:" → 1				
Pail AC Volume Cikin h	1		Vol.AC	Exp			u				
Phil AC, Agy Dry Final	j		V.M.A.	u+y	<u> </u>	二	٧				
RijACAgilmm Loss	k			(4x)×100			Vi			<del> </del>	
Poil AC Ag; Vol. 1-k	1:	<u></u>	BI	100-1	]		у				
Agy Volume	in		Voids	-	<u> </u>	77	<del>,</del> /			<u> </u>	
Ay J. Sp. G. m	r.		1	signed	K	91	1/al	Kos		gate:	8/23
•					/		-	(			•



Date: <u>8/23/90</u>
Phone: 612-779-5614
FAX: 612-779-5580

To: _			, E	ngine	er,					
The r	nix design for Spec. is project as follows:	23	40-5peisl, N	lixtu	те Туре	41	5	is	hereby appr	roved
S	S.P. 9 S.A.P.		-			Ю В	-  M	ΙĮΧ	FORM	ULA
	1 2			11 1				20	)	
	Begin with test num	ber:	Formula	_	Sieve Size		W	orking	Range	
For	(Card 1)		1 100	P	1 1/2"	33	1   0	0	1   0	0 38
Base	1/1 1 1 2 2 2 2 2 1 1 1 1	010	1 . 1	e r	3/4"	39	110	0	100	, 44
	21	27	100	c	5/8"	45	10	0	100	T = (
For	(Card 2)			e n	1/2"	51	19	0	11010	<u>ا</u> _ ر
Binder	4    B   I   4	010		t	3/8"	57	17	8	19 10	
	21 (Card 3)	27		p	#4	63	15		64	<del> </del>
For		. 1	111	a f	#10	69	4		1418	<b></b> -
Leveling	411 L V 410			s s	#40	75	1 /	7	1215	80
	(Card 4)	27	3.6		#200	81	7	.=	161.	_
For Wearing	41/ WE 410	010		L		- 1				
wearing	21	27	6.8	į	% A. C.		<u>6.</u> Min.	<u> </u>	7 · / Max.	
For	(Card 5)		Target (New)				IVIIII.		Max.	
houlders	S   H	1	Use /20 /	150	penetra	tion gra	ade asphal	t.		
	21	27	<u> </u>		J					
	Trial Mix No. 0-			ates a	Marshall	Densit	y of	120	6.2 PC	Fat
	blow	s per side								
	Proportions	<u></u>	Source					B	A or BR#	<del></del>
	4 %	Solbe	rg - Ninining	<u>us</u>	(3/4" R	ock)		0-90	392	
	54 %	Bitumin	nous Roadway	<i>ا</i> حــــــــــــــــــــــــــــــــــــ	(3/4")	Minus	)	0- <i>903</i>	393	
	24 %		-Larson		(1/2" K	ock)		0-903		
	6 %	Trash Depot (Rubber) 0-90397								
	12 %	Omar	,		(Shingle	_	·	0- 904	45	
Rema	arks: Obtain 60#			od 4						
1 CIII	Mix will be	Tende	e until c	00/			/ /			
<del> </del>	1110	,-,	VIII.							
***********										

DO1 14 54303 AC	- 88)			Sheet	of	7.1	M. No.		1011	
						r-	V ()	90	194	<del></del>
				Specific	otigs No	کار	115	Previou	15 Mix No	
Project No. 9PR 100	8 0	30 <b>3</b>	<u> </u>	AB No.			Source			portion
Location RAMSEY		T.H.I	Vo.	90392	504	3ERC	-NiviNI	VOER Y	Y ROCK 4	%
Additive		هو		90393			ROADWAY			%
AC Source ASAN	ANA	2		90394			4 LARSON		24	%
AC Grade				90397			EPOT RUBL		6	0/0
AC Sp.G. c	]		ı	90415	ON	14N	SHINGLE		K	%
Tests.	_	B.I. M	r Ts	Extract			ation (	700	1	
	<del></del>					rau	1		<del></del>	
,		MIX A	46003	Guit	: 8/21	٠,٠	MIXZ	3	date	;
	3.5			# Blows	· <0		WI AC D	6.5	# Blows	50
Marshall ID	1	/	7_	3			NACE.	<i>7</i> .	3	
Height -	a	2 /32	7 15	2 1/6		a	2 13/2	2 31/4	2 3364	
Dry Wght.	A	1000.7	1004.8	1007.9		A	1015.1	1010.4	1009.5	
SSD Want.	В	1003.4	1009.4	1014.1		В	1019.6	1012.3	1013-1	
Imm. Wght.	C	497,5	506.2	511.1		C	523.0	514.1	510.1	1
Volume B-C	Ď	505.9				D	496.6	498.2	503.0	
Bulk Sp.G. AD	E	1.978	1.997	2.004	1.993	Ε	2.044	2.028	2.006	2.026
Density Ex 62.3	F				124.1	F	<u> </u>			126.2
Flow		25	29	26		<b>†</b>	126	29	22	
Stabilit Dial	1	270	35O	380		1	326	270	270	
Stability Dial chart ht. Stability		275	357	380	337		326	271	271	289
	1	!					1	1		
Container ID		$\mathcal{B}$			·	1	<u></u>			<u> </u>
Cont. + Sa Dry	K					K				
Cont. Dry	L	912.5				<u> </u>	742.1		<del> </del>	ļ
So, Dry K-L	M	2020.3				M	2024.7			
Cont + So Imm.	_ N	1426.2				N	1252.5			
Cont. Imm,	I <u>P</u>	327.4				ΙĎ	162.1	***************************************		ļ
Sa Imm N-P	Q	1098.8				Q	10904		<b></b>	
Sa Volume M - Q	R	921.5	<u> </u>			R	934.3		<u> </u>	
Max. Sp. G. MR	S	2.192			2.192	S	2.167		ļ	2167
Rice Voids 100x(S-E	) T				9.0	T				65
1116 YV143 1 2 3		2355					2.352.			
Pan, Agg. Dry	d	Target	AC ratio	b/c		þ				1935
Pun Dry	e	setu	Agg ratio	100-b)/n		<u>]</u> q_				1161.6
Agg Dry d-e	f	Day Act		p + q		r				
Pail, AC Dry	g	SSD	TM Sp.G.	100/r		s				
Pail AC Imm	h	Imm	VE MIX	(E/s)× 100		1	]		<b></b>	
Pail AC Volume 9 - h	] [	DRig	Vol.AC	Exp		U				
Riil, AC, Ag, Dry	Ţj	Final	V. M,A,	u+y	<u> </u>	v	L			<b></b>
RijACAg , Imm	k	Loss	V.F. Agg.	(4x)×100		W				
PhilACAG; Vol. j-k	1		BI	100-1		у				
Agy Volume 1 - i	ın	90 Loss	Voids	100-1	L_,,	<u></u>	1,	<u>L</u>	<u> </u>	ــــــــــــــــــــــــــــــــــــــ
Agj. Sp. G. /m	r.		1	Signed	60	/1/10	lhon		date:	8/23
1241, Ab. 2. 1		<u> </u>	<b>4</b>	<u></u>	(					7



Date: 8/23/90 Phone: 612-779-5614 FAX: 612-779-5580

# 0-

To: _		, Engineer,								
	nix design for Spec. is project as follows:		40 Spuis	Mixt	ure Type	4	45	i	s hereby appro	ved
S	S.P. 9 S.A.P.					ОВ	-  M		FORMU	LA
	1 2	1	Y1-	11	12		•	_	20 - Dansa	
	Begin with test num	ber.	Formula	۱ - ۳۰	Sieve Size	1	<del> </del>		Range	٦
For	(Card 1)		100	P e	1 1/2"	33	$1 \mid 0$	10	1   0   0	<b>-</b> 1
Base			100	r	3/4"	39	10	0	100	44
	21 (Card 2)	27	1.00	c e	5/8"	45	110	10	100	50
For		010	96	n	1/2"	51	19	10	100	56
Binder	4   B I 5  21	27	82	t	3/8"	57	17	16	1818	62
	(Card 3)		53	p	#4	63	4	17	1519	68
For Leveling	411LV5	010	41	a s	#10	69	13	317	1415	74
Devening	21	27	Z2	S	#40	75		118	2 6	80
For	(Card 4)		3.9		#200	81	2	7   •	141.	86
Wearing		00	3.0		% A. C.		2	7	3.3	7
	21	27	Target (New	' ')			2. Mii	n.	Max.	٦
For	(Card 5)			·		•		•		
Shoulders	SH		Use 120	1150	penetra	tion gra	ade asph	ait.		
	21	27					_		de a	
	Trial Mix No. 0-		indi	icates	a Marshall	Densit	y of		PCF	at
	blow	's per side.	Cours	f l	Material			1	BA or BR#	
	Proportions							1		
	7 %	Solberg.	- Nininger	(	74 Rock	<del>)</del>			0392/464	-
	3/10	Bituminous	Resduzys		-14 Minus	)		1 _	0393/465	_
	27 %	Shiely -	Laeson		12' Rock)			<del> </del>	394/463	_
	9 %	Oman		(3	hingles)			0- 90	0415	
	%							0-		
Rema	arks: Obtain 60 5	ample for	Maplewood	1 6	6 % C.Tu	Rgcon/	IR.Wall	652		
		<u>'</u>					<b></b>	•		
							<u></u>	<u></u>		
Approv	cd by:			cc:						
**	•				at'ls Engr. (Dist.	) (2)				

Ft. Snelling (3) Contractor

DOT TP 24							1.	v. No.	0.90	195	
					Specifico	tian No.		415	Previou	isMixNo.	
Project No.	9 PR H	208	0303		AB No.			Source	-/ A	Propo	cti
Location R	ensey (B	ike	34 )IHI	10.	9046			-Niain inger	3/4 Koc	k 7	
Additive			26		46.			duys	44 M.		
AC Source	Ashland				46: 39		יעף.	Depot Ru	1/2 Roc	k 27	
AC Grade	1.017	0			41			an Shingle		9	
AC Sp.G. C	7.017				and the second second					· · · · · · · · · · · · · · · · · · ·	-
Tests			B.I. M	Js	Extracti	on G	rad	ation 10	ther		
			IMIX C	,	<del>d</del> ate:			MIX		datet	
			wt. AC	366.0		<i>-</i> .		W! AC		## O.	
		3.0	% AC b		# Blows	50		% AC b		# Blows	
Marshall ID		<del> </del>	2 29/4	2 29/4	7 15/12		а			-	
Height	- AN MARK	Δ.	11-11	1175.4	1174 4						
Dry Want. SSD Want.		B	1177.2	1176.3	1176.3		A B				
Imm. Waht.	t in a transmission of annual ten	C	675,3	675.7	674.2		С				
Volume Volume	B-C	Ď	501.9	500.6	502,1		D				فاد يججو
	A/D	E	2343	2.348	2.339	2.343	Ε				
Bulk Sp.G.		1 =	-315	2.0,0	1	146.0	F	<b></b>			-
Density	E × 62.3	F			<u> </u>	176.0		1		<u> </u>	در پینده. 
Flow		1	15	12.5	77					1	
StabilityDial			2510	2370	2320						
StabilityDial Stability	chart ht. x cor		2585	2441	2366	2464	1				الانجينة
Oldon riy		- <del></del>		1			!			1	
Container ID			A	ļ							
Cont + Sa Dry		K	01/21				K			_	
Cont. Dry	1/ 1		943.1			anganan agamatan tertekan ara-	М				
So. Dry	K-L	M	1				N				
Cont + So Imm		P	1		i	~ · · · · ·	P		and the second		
Cont. Imm Sa Imm			<del></del> 1				Q				
Sa Volume	M - Q	F					J R				
}	MR	S	T ',			2.424	s				
Max. Sp. G.	S-F	1				3.3	T				
Rice Voids	100×(25)	4 '	1		<u> </u>		<b>┦</b> —		1		عربيط
ĺ		i d	2.553	AC ratio	<b>b</b> /c	223.9	P			T	
Rin, Ago Dry Run Dry	set 6	e		Aga ratio	100-b/n	1343.4	ų				tr
Run Dry Agg Dry	Day Act.	f	- T		p + q		r		ļ		
Pril AC Dry		g		TM Sp.C	100/		_ s	-			
Fail AC Imm	Imm	1		VE MIX	( (E/s)× 100					_	
Phil AC Volume	Overa			Vol.AC	Exp		- u			-	-
Ril, AC, Agg Dry	tinal		<u>l</u>	V.M.A.			<b>-</b>		1		-
RiJAC, Ag , Imr				V.F. Agg			7				Г
PhilACA9; Vol	j-k	< I	n	Voids	100-1	<u></u>			<u> </u>		L.
Agy. Volume	% total	<del>)  </del> -		7	signed	(.)(1		lkosi		gate:	51

DOT TP 24303A(1-48)	Sheet	of	T.N	1. No.	90	195	
	Specifica		T	// S		ıs Mix No.	
Project No. 9PR 1008 03 03	AB No.	11011.10	. 21	Source	Market Mark		artion
Lacotion RAMSEY (BIKE PATH) T.H. No.	90392	50	LBER	6 NININ	GER 3		
Additive 96	90393			POADWAY			
AC Source ASHLAND	90394	SHI	ELY	LARSON	1/2 Rock	27	
AC Grade 120/150	90397			DEPOT RE			
AC Sp.G. C	415			SHINGLE		9	1/0
Tests B.I. Mr Ts	Extraction	on G	rado	tion C	Other		
· I INAIV A	date:	8/21		MIX Z	3620.0	date	:8/2/
$\mu$ 6.7 wt. $\Delta C = \frac{493}{100}$			22.3	wt AC	620.0		
4.0 % AC b 4.25		50	5.0	% AC b		# Blows	50
Marshall ID / 2	39/			10/	7 10/	2 10/2	
Height 0 2 932 2 19	04 - 32-		G	2/32	1083-7	1086.6	
Dry Wght. A 1096.8 1095			A B	1081.5	1084-1	1086.9	
SSD Wgh 1. B 1097.8 1096			C	614.2	617-6	621-1	
Imm. Wght. C 633.7 630.			D	468.0	466.5	465.8	
VOIDING 199				ľ			
Bulk Sp.G. $\frac{A}{D}$ E 2.363 2.43	8 2.340	2.380	E	2.311	2.323	2.333	2.322
Density   E×62.3   F		148.2				<u> </u>	1.7.76
Flow 13 15	15		ļ	17	19	17	
Stability Dial 1880 1610	>   1350		ļ	800	1020	960	
Stability Dial chart ht. Stability Chart cor.		1775		1	<u> </u>		1020
Container ID D				E			
Cont. Sa Dry K			K				
Cont. Dry L 767.2			L	780.1		<del> </del>	
Sa. Dry K-L M 2041.9			M	2034.2			
Cent+Salmm N 1386.7			N	1364.1			
Cont.   mm,   P   185.4			Þ	196.4			+
Sa Imm N-P Q /20/.3			Q R	1168.7			
Sa Volume M - Q R 840.6			1				<b></b>
Max. Sp. G. M/R S 2.429 74	7	2429		2.350	2363	-	2.35
Rice Voids 100x(S-E) T 1.5	5	2.0	T		1.7		tool
2.50 17, 50	, d		1	2.558			
Pan, Agg Dry d AC ra	ilio b/c		- P		<del> </del>		
Pın Dry e Aggr	atic (100-b)/n	ļ	4.				
1 De d-e f	p+q			-	<del> </del>		<b></b>
TMS	p.G. 100/r	<b> </b>	S.	-	<del> </del>		_
Fair AC Imm h VE M	11X LE/SIX IOU	<b> </b>	<del>                                     </del>	<del> </del>	<del> </del>		<del>                                     </del>
Pail AC Volume 9 - h [ Vol. A		<del>                                     </del>	- u	<del>                                     </del>			
Pril AC And Dry	16. 1 400	-	-4 "	-	1		
BitACAy, Imm			7	-	<del> </del>		1
Hail AC Ag; Vol. 1-k			y				<u> </u>
Agy Volume		(2//	1, 1	lkoss		date	8/2
Ag, Sp.G. /m r.	signed	NY	va	Wass		Juie	

# STATE OF MINNESOTA DEPARTMENT OF TRANSPORTATION

# TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY TESTS COMPLETED SUBMITTED BY INSPECTOR DATE SAMPLED DATE RECEIVED-	ST. PAUL 01/26/91 C TURGEON 01/07/91	REPORT DATE PROJECT NUMBER- TYPE OF CONST COURSE STATION NO	FEB 01, 199 9PR6002 2341	91
DATE RECEIVED-	01/07/91	FIELD ID	2,3,4	
	, , , = =		21313	

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS

COMMENTS: BIKEPATH 2,3,4

TEST RESULTS

# SIEVE ANALYSIS (SQUARE OPENINGS)

#### % PASSING

PASS 1 IN. SIEVE PASS 3/4 IN. SIEVE		BITUMEN(%) 6.2 MOISTURE(%)
PASS 5/8 IN. SIEVE	100.0	VOLATILE(%)
•	97.0	TESTS ON RECOVERED ASPHALT
PASS 3/8 IN. SIEVE	81.0	PENETRATION 77 F 76
PASS #4 SIEVE	50.0	DUCTILITY 77 F. (CM)
PASS #10 SIEVE	40.0	SOFTENING POINT, F.
PASS #20 SIEVE	32.0	KVISC 275 F. CS
PASS #40 SIEVE	24.0	AVISC 140 F. POISES 1153
PASS #80 SIEVE	9.0	RICE VOIDS
PASS #100 SIEVE	8.0	DENSITY LBS PER CU FT
PASS #200 SIEVE	5.8	FLOW
		STABILITY

REMARKS: 3% CRUMB RUBBER

COPIES TO:	CHARGE NO.:	THIS REPORT INTENDED ONLY FOR
		INFORMATION AS TO UNIFORMITY OF
BIT. OFFICE	1125	PRODUCTION MAKE NO CHANGES IN
	1020	RECOMMENDED ASPHALT PERCENTAGE
	1111	WITHOUT CONTACTING BITUMINOUS ENGINEER.

SIGNED-----

### STATE OF MINNESOTA DEPARTMENT OF TRANSPORTATION

# TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY TESTS COMPLETED SUBMITTED BY	ST. PAUL 01/26/91 C TURGEON	REPORT DATE PROJECT NUMBER- TYPE OF CONST	FEB 01, 1991 9PR6002 2341
INSPECTOR		COURSE	
DATE SAMPLED		STATION NO	
DATE RECEIVED-	01/07/91	FIELD ID	6,7,8

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS

COMMENTS: BIKEPATH 6,7,8

TEST RESULTS

# SIEVE ANALYSIS (SQUARE OPENINGS)

#### % PASSING

PASS 1 IN. SIEVE PASS 3/4 IN. SIEVE			BITUMEN(%) 5.3 MOISTURE(%)
PASS 5/8 IN. SIEVE	100.0		VOLATILE(%)
PASS 1/2 IN. SIEVE	96.0		TESTS ON RECOVERED ASPHALT
PASS 3/8 IN. SIEVE	83.0		PENETRATION 77 F 52
PASS #4 SIEVE	58.0		DUCTILITY 77 F. (CM)
PASS #10 SIEVE	47.0		SOFTENING POINT, F.
PASS #20 SIEVE	39.0		KVISC 275 F. CS
PASS #40 SIEVE	28.0		AVISC 140 F. POISES 2125
PASS #80 SIEVE	11.0	•	RICE VOIDS
PASS #100 SIEVE			DENSITY LBS PER CU FT
PASS #200 SIEVE	10.5		FLOW
			STABILITY

#### REMARKS:

COPIES TO:	CHARGE NO.:	THIS REPORT INTENDED ONLY FOR
		INFORMATION AS TO UNIFORMITY OF
BIT. OFFICE	1125	PRODUCTION MAKE NO CHANGES IN
	1020	RECOMMENDED ASPHALT PERCENTAGE
	1111	WITHOUT CONTACTING BITUMINOUS ENGINEER.

# STATE OF MINNESOTA DEPARTMENT OF TRANSPORTATION

# TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY TESTS COMPLETED SUBMITTED BY INSPECTOR	ST. PAUL 01/29/91 C TURGEON	REPORT DATE PROJECT NUMBER- TYPE OF CONST COURSE	FEB 01, 1991 9PR6002 2341
DATE SAMPLED DATE RECEIVED-	01/07/91	STATION NO FIELD ID	10,11,12

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS

COMMENTS: BIKEPATH 10,11,12

TEST RESULTS

## SIEVE ANALYSIS (SQUARE OPENINGS)

#### % PASSING

PASS 1 IN. SIEVE PASS 3/4 IN. SIEVE		BITUMEN(%) 7.8 MOISTURE(%)
PASS 5/8 IN. SIEVE	100.0	VOLATILE (%)
PASS 1/2 IN. SIEVE	94.0	TESTS ON RECOVERED ASPHALT
PASS 3/8 IN. SIEVE	81.0	PENETRATION 77 F 111
PASS #4 SIEVE	57.0	DUCTILITY 77 F. (CM)
PASS #10 SIEVE	44.0	SOFTENING POINT, F.
PASS #20 SIEVE	34.0	KVISC 275 F. CS
PASS #40 SIEVE	24.0	AVISC 140 F. POISES 268
PASS #80 SIEVE	9.0	RICE VOIDS
PASS #100 SIEVE	8.0	DENSITY LBS PER CU FT
PASS #200 SIEVE	5.7	FLOWSTABILITY

REMARKS: 6% CRUMB RUBBER

COPIES TO:	CHARGE NO.:	THIS REPORT INTENDED ONLY FOR INFORMATION AS TO UNIFORMITY OF
BIT. OFFICE	1125 1020 1111	PRODUCTION MAKE NO CHANGES IN RECOMMENDED ASPHALT PERCENTAGE WITHOUT CONTACTING BITUMINOUS ENGINEER.

### STATE OF MINNESOTA DEPARTMENT OF TRANSPORTATION

# TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY TESTS COMPLETED SUBMITTED BY INSPECTOR		REPORT DATE PROJECT NUMBER- TYPE OF CONST	FEB 01, 1991 9PR6002 2341
DATE SAMPLED DATE RECEIVED-	01/07/91	COURSE STATION NO FIELD ID	C TURGEON

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS

COMMENTS: BIKEPATH 14,15,16

TEST RESULTS

SIEVE ANALYSIS (SQUARE OPENINGS)

# % PASSING

PASS 1 IN. SIEVE		BITUMEN(%) 5.4
PASS 3/4 IN. SIEVE		MOISTURE(%)
PASS 5/8 IN. SIEVE	100.0	VOLATILE(%)
•	96.0	TESTS ON RECOVERED ASPHALT
,	84.0	PENETRATION 77 F 34
PASS #4 SIEVE	56.0	DUCTILITY 77 F. (CM)
PASS #10 SIEVE	46.0	SOFTENING POINT, F.
PASS #20 SIEVE	38.0	KVISC 275 F. CS
PASS #40 SIEVE	27.0	AVISC 140 F. POISES
PASS #80 SIEVE	13.0	RICE VOIDS
PASS #100 SIEVE	11.0	DENSITY LBS PER CU FT
PASS #200 SIEVE	7.8	FLOW
		STABILITY

REMARKS: 9% SHINGLES

COPIES TO:	CHARGE NO.:	THIS REPORT INTENDED ONLY FOR
		INFORMATION AS TO UNIFORMITY OF
BIT. OFFICE	1125	PRODUCTION MAKE NO CHANGES IN
	1020	RECOMMENDED ASPHALT PERCENTAGE
	1111	WITHOUT CONTACTING BITUMINOUS ENGINEER.

SIGNED-----

ASST. CHIEF CHEMIST

# STATE OF MINNESOTA DEPARTMENT OF TRANSPORTATION

#### TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY	ST. PAUL	REPORT DATE	FEB 01, 1991
TESTS COMPLETED	01/26/91	PROJECT NUMBER-	9PR6002
SUBMITTED BY	C TURGEON	TYPE OF CONST	2341
INSPECTOR		COURSE	
DATE SAMPLED		STATION NO	
DATE RECEIVED-	01/07/91	FIELD ID	18,19,20

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS

COMMENTS: BIKEPATH 18,19,20

TEST RESULTS

# SIEVE ANALYSIS (SQUARE OPENINGS)

#### % PASSING

PASS 1 IN. SIEVE	100.0	BITUMEN(%) 7.3
PASS 3/4 IN. SIEVE	100.0	MOISTURE(%)
PASS 5/8 IN. SIEVE	99.0	VOLATILE(%)
PASS 1/2 IN. SIEVE	96.0	TESTS ON RECOVERED ASPHALT
PASS 3/8 IN. SIEVE	81.0	PENETRATION 77 F 55
PASS #4 SIEVE	52.0	DUCTILITY 77 F. (CM)
PASS #10 SIEVE	41.0	SOFTENING POINT, F.
PASS #20 SIEVE	33.0	KVISC 275 F. CS
PASS #40 SIEVE		AVISC 140 F. POISES 2548
PASS #80 SIEVE		RICE VOIDS
PASS #100 SIEVE	8.0	DENSITY LBS PER CU FT
PASS #200 SIEVE	5.5	FLOW STABILITY

REMARKS: 3% C RUBBER 6% SHING

COPIES TO:	CHARGE NO.:	THIS REPORT INTENDED ONLY FOR INFORMATION AS TO UNIFORMITY OF PRODUCTION MAKE NO CHANGES IN
BIT. OFFICE	1125 1020 111	RECOMMENDED ASPHALT PERCENTAGE WITHOUT CONTACTING BITUMINOUS ENGINEER.
		SIGNED